



Base Shear = 502.1 (kips)  
 Moment @ C.G. = 16897.0 (k-ft)

Checks for Unfactored Load Stresses

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 NOTE: This check is not performed for mild reinforced members.

Checks for Ultimate (Design) Moment by Strain Compatability

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 Load Case 2: 0.900\*DL+1.600\*WIND C= 34.4(in) fconcr= 1404.0(k)  
 WIND Mu @ C.G.= 27035.2(k-ft) phi= 0.900  
 phi\*Mn @ C.G.= 28498.7(k-ft)

Checks for Ultimate (Design) Shear at Horizontal Joint

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 Load Case 2: 0.900\*DL+1.600\*WIND  
 Factored Applied Shear Vu = 803.4(k)  
 0.75\*As\*fy\*Friction Factor = -722.5(k)  
 Use: -----  
 Req'd horiz. jt. connections capacity = 80.8(k)/phi

Summary of Concrete and Bar Forces at Provided Ultimate

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Load Case 2:			
	Force*	Bar	Location
	(kips)	Strain	(in)
Compr. block =	-1404.04		294.23
Bar( 1) =	187.20	0.02325	6.00
Bar( 2) =	187.20	0.02273	12.00
Bar( 3) =	187.20	0.02221	18.00
Bar( 4) =	187.20	0.02168	24.00
Bar( 5) =	36.00	0.01785	68.00
Bar( 6) =	36.00	0.00947	164.00
Bar( 7) =	19.27	0.00110	260.00
Bar( 8) =	-91.19	-0.00100	284.00
Bar( 9) =	-139.04	-0.00152	290.00
Bar(10) =	-186.89	-0.00204	296.00
Bar(11) =	-187.20	-0.00257	302.00

\*NOTE: Negative forces indicate compression.

NOTE: Dry pack/grout must have a strength equal to f'c= 5.000(ksi).

Panel Shear

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L.C.	Vu (k)	Mu (k-ft)	Nu (k)	d (in)	Vc Provided		
					(11-3) (k)	(11-29) (k)	(11-30) (k)
2	803.4	27035.2	1169.9	246.40	418.2	923.9	678.4
	Vc (k)	Vu/phi (k)	ACI Section		Avh (in-2/ft)	Avn (in-2/ft)	
2	678.4	1071.1	11.10		0.36	0.36	

NOTE: Panel shear reinforcing assumes no openings.

NOTE: Minimum spacing is 18.00 in.

\*\*\*\*\* MOMENT REVERSAL \*\*\*\*\*

Lateral Loads: (WIND)

Base Shear = 502.1 (kips)  
 Moment @ C.G. = -16897.0 (k-ft)

Checks for Unfactored Load Stresses

NOTE: This check is not performed for mild reinforced members.

Checks for Ultimate (Design) Moment by Strain Compatability

-----  
 Load Case 2: 0.900\*DL+1.600\*WIND C= 34.8(in) fconcr= 1421.5(k)  
 WIND Mu @ C.G.= -27035.2(k-ft) phi= 0.900  
 phi\*Mn @ C.G.= -29705.3(k-ft)

Checks for Ultimate (Design) Shear at Horizontal Joint

-----  
 Load Case 2: 0.900\*DL+1.600\*WIND  
 Factored Applied Shear Vu = 803.4(k)  
 0.75\*As\*fy\*Friction Factor = -722.5(k)  
 Use: -----  
 Req'd horiz. jt. connections capacity = 80.8(k)/phi

Summary of Concrete and Bar Forces at Provided Ultimate

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 Load Case 2:  
 Force\* Bar Location  
 (kips) Strain (in)  
 Compr. block = -1421.46 13.94  
 Bar( 1) = -187.20 -0.00256 6.00  
 Bar( 2) = -186.52 -0.00204 12.00  
 Bar( 3) = -139.32 -0.00152 18.00  
 Bar( 4) = -92.12 -0.00101 24.00  
 Bar( 5) = 36.00 0.00278 68.00  
 Bar( 6) = 36.00 0.01104 164.00  
 Bar( 7) = 36.00 0.01930 260.00  
 Bar( 8) = 187.20 0.02137 284.00  
 Bar( 9) = 187.20 0.02188 290.00  
 Bar(10) = 187.20 0.02240 296.00  
 Bar(11) = 187.20 0.02291 302.00

\*NOTE: Negative forces indicate compression.

NOTE: Dry pack/grout must have a strength equal to f'c= 5.000(ksi).

Panel Shear

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L.C.	Vu	Mu	Nu	d	(11-3)	Vc Provided	
	(k)	(k-ft)	(k)	(in)	(k)	(11-29)	(11-30)
						(k)	(k)
2	803.4	27035.2	1169.9	246.40	418.2	923.9	678.4
	Vc	Vu/phi	ACI Section		Avh	Avn	
	(k)	(k)			(in-2/ft)	(in-2/ft)	
2	678.4	1071.1	11.10		0.36	0.36	

NOTE: Panel shear reinforcing assumes no openings.

NOTE: Minimum spacing is 18.00 in.